

## CUSTOMER SUPPORT NOTE

# Calculation of spring constants for use as spring supports in LUSAS piling analyses

|              |                       |
|--------------|-----------------------|
| Note Number: | <b>CSN/LUSAS/1001</b> |
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This support note is issued as a guideline only.



Forge House, 66 High Street, Kingston upon Thames, Surrey, KT1 1HN, UK  
Tel: +44 (0)20 8541 1999 Fax: +44 (0)20 8549 9399  
Email: [info@lusas.com](mailto:info@lusas.com) [www.lusas.com](http://www.lusas.com)

# Table of Contents

|    |              |   |
|----|--------------|---|
| 1. | INTRODUCTION | 1 |
| 2. | DESCRIPTION  | 1 |
| 3. | REFERENCES   | 3 |

## 1. Introduction

This support note describes how lateral spring constants should be derived from Standard Penetration Test (SPT) “N” values for use in LUSAS piling analyses where the soil is not modelled explicitly and no information is known regarding the soil stiffness. SPT “N” values are used in the calculation of stress-strain modulus,  $E_s$  which in turn is then transformed into a lateral modulus of subgrade reaction,  $k_s$ . Once a  $k_s$  profile has been determined spring constants can be calculated based on nodal spring spacing within the finite element mesh.

## 2. Description

When calculating spring constants to be used as lateral restraints in piling analyses the initial step is to calculate a soil profile for the lateral modulus of subgrade reaction,  $k_s$ . This itself can be derived from the stress-strain modulus of the soil,  $E_s$  (Glick, 1948).

$$k_s' = \frac{22.4E_s(1-\mu)}{(1+\mu)(3-4\mu)[2\ln(2L_p/B) - 0.433]}$$

where:

|       |                         |
|-------|-------------------------|
| $E_s$ | = stress-strain modulus |
| $\mu$ | = Poisson's ratio       |
| $L_p$ | = pile length, m        |
| $B$   | = pile width, m         |

Then  $k_s = \frac{k_s'}{B}$

An approximate value of stress-strain modulus,  $E_s$ , can be derived from results of Standard Penetration Test “N” values or “blow-counts”.

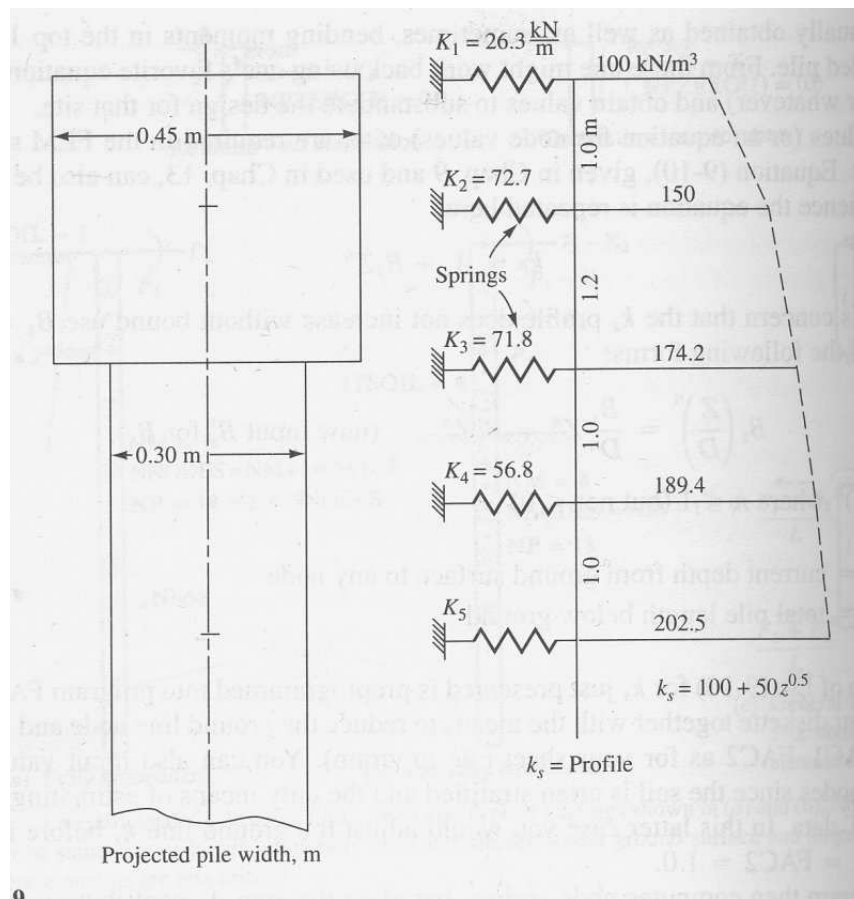
The following table provides the empirical relationship to be used depending on the soil type under consideration (Bowles, 1996).

| Soil type                      | $E_s$ (kPa)  |
|--------------------------------|--------------|
| Sand (normally consolidated)   | 500 (N + 15) |
| Sand (saturated)               | 250 (N + 15) |
| Gravelly sand                  | 1200 (N + 6) |
| Clayey sand                    | 320 (N + 15) |
| Silts, sandy silt, clayey silt | 300 (N + 6)  |

A check on the values of lateral modulus of subgrade reaction obtained can be quickly made by comparing the values against typical values such as those found in the following table for various soil types.

| Soil type                | $k_s$ (MN/m <sup>3</sup> ) |
|--------------------------|----------------------------|
| Dense sandy gravel       | 220 - 400                  |
| Medium dense coarse sand | 157 - 300                  |
| Medium sand              | 110 - 280                  |
| Fine or silty, fine sand | 80 - 200                   |
| Stiff clay (wet)         | 60 - 220                   |
| Stiff clay (saturated)   | 30 - 110                   |
| Medium clay (wet)        | 39 - 140                   |
| Medium clay (saturated)  | 10 - 80                    |
| Soft clay                | 2 - 40                     |

Actual spring constants to be used in the analysis are derived from the lateral modulus of subgrade reaction and nodal spacing down the length of the pile as shown in the following example.



**Example 16-9.** Compute the first four node springs for the pile shown in Fig. E16-9. The soil modulus is  $k_s = 100 + 50Z^{0.5}$ . From the  $k_s$  profile and using the average end area formula:

$$K_i = \frac{BL}{6}(2k_{s,i} + k_{s,i-1}) \quad \text{or} \quad \frac{BL}{6}(2k_{s,i} + k_{s,i+1})$$

$$K_1 = H(1) \times B(1)(2k_{s,1} + k_{s,2})/6 = 1.0 \times 0.45(2 \times 100 + 150)/6 = 26.3$$

$$K_2 = H(1) \times B(1)(2k_{s,2} + k_{s,1})/6 = 1.0 \times 0.45(2 \times 150 + 100)/6 = 30.0$$

$$K'_2 = H(2) \times B(2)(2k_{s,2} + k_{s,3})/6 = 1.0 \times 0.45(2 \times 150 + 174.2)/6 = 42.7$$

$$K_3 = H(3) \times B(3)(2k_{s,3} + k_{s,2})/6 = 1.2 \times 0.45(2 \times 174.2 + 150)/6 = 44.9$$

$$K'_3 = 1.0 \times 0.30(2 \times 174.2 + 189.4)/6 = 26.9$$

$$K_4 = 1.0 \times 0.30(2 \times 189.4 + 174.2)/6 = 27.7$$

**Summary.**

$$K_1 = 26.3 \text{ kN/m}$$

$$K_2 = K_2 + K'_2 = 30.0 + 42.7 = 72.7 \text{ kN/m}$$

$$K_3 = K_3 + K'_3 = 44.9 + 26.9 = 71.8 \text{ kN/m}$$

$$K_4 = 27.7 + 29.1 = 56.8 \text{ kN/m, ... , etc.}$$

### 3. References

GLICK, G. W. (1948). "Influence of Soft Ground in the Design of Long Piles". 2<sup>nd</sup> ICSMFE, vol. 4, pp. 84-88.

BOWLES, J. E. (1996). "Foundation Analysis and Design". 5<sup>th</sup> Edition, McGraw-Hill, Singapore.